

# COST-BENEFIT ANALYSIS OF SEQUENTIAL WARNING LIGHTS IN NIGHTTIME WORK ZONE TAPERS

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Report to the Smart Work Zone Deployment Initiative



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16. Abstract Improving safety at nighttime work zones is important because of the extra visibility concerns. The deployment of sequential lights is an innovative method for improving driver recognition of lane closures and work zone tapers. Sequential lights are wireless warning lights that flash in a sequence to clearly delineate the taper at work zones. The effectiveness of sequential lights was investigated using controlled field studies. Traffic parameters were collected at the same field site with and without the deployment of sequential lights. Three surrogate performance measures were used to determine the impact of sequential lights on safety. These measures were the speeds of approaching vehicles, the number of late taper merges and the locations where vehicles merged into open lane from the closed lane. In addition, an economic analysis was conducted to monetize the benefits and costs of deploying sequential lights at nighttime work zones. The results of this study indicates that sequential warning lights had a net positive effect in reducing the speeds of approaching vehicles, enhancing driver compliance, and preventing passenger cars, trucks and vehicles at rural work zones from late taper merges. Statistically significant decreases of 2.21 mph mean speed and 1 mph 85% speed resulted with sequential lights. The shift in the cumulative speed distributions to the left (i.e. speed decrease) was also found to be statistically significant using the Mann-Whitney and Kolmogorov-Smirnov tests. But a statistically significant increase of 0.91 mph in the speed standard deviation also resulted with sequential lights. With sequential lights, the percentage of vehicles that merged earlier increased from 53.49% to 65.36%. A benefit-cost ratio of around 5 or 10 resulted from this analysis of Missouri nighttime work zones and historical crash data. The two different benefit-cost ratios reflect two different ways of computing labor costs.			
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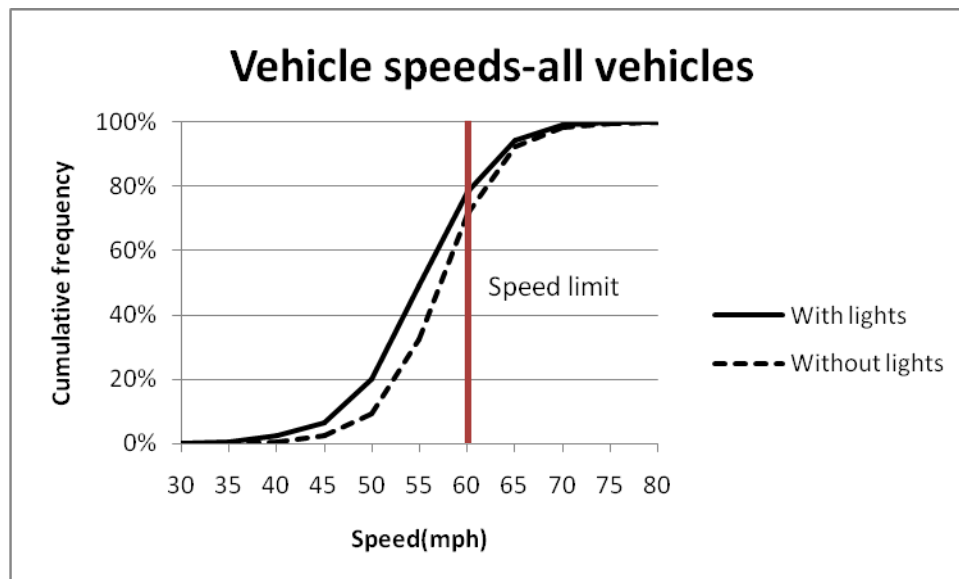
## EXECUTIVE SUMMARY

Sequential warning lights were evaluated using three measures of safety performance derived from three different video fields-of-view. The radar view yielded individual vehicle speeds and speeds of vehicles not following in platoons. The near taper view produced transverse vehicle position at the taper. The far taper view produced closed lane occupancy at different zones approaching the taper. The video footages were collected from three different nighttime work zones located on I-70 in Missouri in both urban and rural areas. Data was collected between 9:30 pm and 1:10 am at each site. The data collected encompassed both passenger cars and trucks. The speed limit for all three sites was 60 mph.

Different speed characteristics were analyzed statistically and found to be significant. In general, speed statistics improved with the deployment of sequential lights. **TABLE I** shows a summary of the speed statistics. Mean speeds decreased from 57.8 mph to 55.6 mph, 85% speeds decreased from 63 mph to 62 mph and the speed compliance rate went up from 71.4% to 78.1%. The overall shape of the speed distribution shifted left, meaning overall speeds have decreased. **FIGURE I** shows how the overall speed distribution has improved with sequential lights. The speed distributions shifted left for both passenger cars and trucks and at both rural and urban work zones. That effect was more pronounced at the urban work zone than at rural work zones. However, speed standard deviation increased from 5.75 to 6.66 mph. The reason for the increase in standard deviation was probably due to a small proportion of drivers who overtook more aggressively near the taper because the taper became more visible. Other measures of performance also support this explanation. The statistical tests applied were the t and z tests for mean and 85% speeds, the F test for speed variability, and the Mann-Whitney and the Kolmogorov-Smirnov for speed distributions.

**TABLE I Speed characteristics**

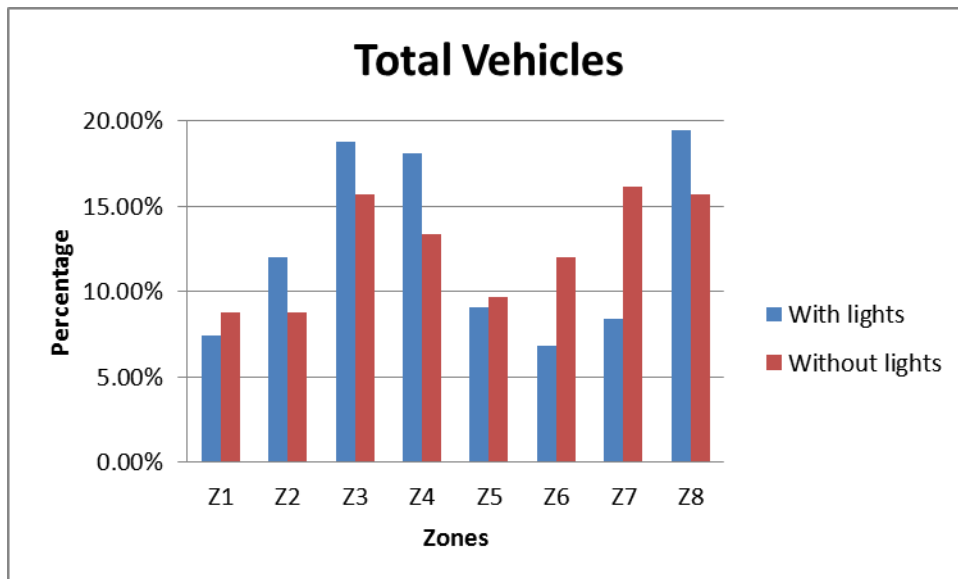
	With Lights	W/o lights	Change
Mean speed (mph)	55.55	57.76	-2.21
85% speed (mph)	62	63	-1.0
Compliance (%)	78.1	71.4	+6.7
Standard deviation (mph)	6.66	5.75	+0.91



**FIGURE I Cumulative speed distributions comparisons: with and without sequential lights.**

The near taper view was used to analyze potential conflicts that could result from last minute merges at the taper. The transverse position of each vehicle was classified as open lane, middle (i.e. between lanes) or closed lane. The rural and urban produced opposite results. In rural work zones, the percentage of vehicles in middle and close positions decreased from 10.7% to 4.9% with sequential lights. But in the urban work zone, the percentage of vehicles in middle and close positions increased from 15.2% to 27.9% with sequential lights. This again supports the explanation that a subset of more aggressive drivers merged near the taper because the taper became more identifiable with sequential lights. This late merging and last minute over-taking behavior was more common in the urban environment because of the higher amount of traffic.

Despite the aforementioned issue at the taper, the overall merging behavior improved with sequential lights. Using the far taper view, 80 ft virtual zones were created upstream of the taper. The zone at which a vehicle merged from the closed lane to the open lane was recorded. The use of sequential lights produced a significant shift in vehicles merging further away from the taper. **FIGURE II** shows the shift of vehicles merging in Zones 5-7 to Zones 1-4. The merging characteristics in Zone 8 are consistent with the near taper view and supports the explanation of the subset of aggressive drivers.



**FIGURE II Percentage of vehicles merging at different zones.**

The benefits and costs associated with the deployment of sequential lights were quantified and monetized using commonly accepted economic analysis methods as documented in the AASHTO Redbook. Nilsson’s crash model was used to estimate improvements in safety from the reduction in speeds. The total annual benefits were estimated at \$3.65 million. The total annual costs were estimated at \$705,008 or \$341,580, depending on how labor was computed.

These estimates assumed that sequential lights were deployed on all interstates and major highways in Missouri. The resulting benefit-cost ratio was around 5 or 10 and the cost effectiveness was around \$25,000 or \$12,000 per injury. Labor costs were the largest component of deployment cost.

In summary, sequential lights appear to be an effective tool for improving driver awareness of the work zone taper. Most measures of performance support this conclusion since speeds were reduced and merge distances increased. A small percentage of aggressive drivers caused an increase in speed variability and late merges. No operational or synchronization problems were observed in the lab or in the field. The economic analysis showed that sequential lights are a cost-effective safety counter measure.

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## **INTRODUCTION**

### **What Are Sequential Warning Lights?**

Sequential warning lights are lights designed to dynamically enhance the visibility of the work zone entrance and to improve driver lane discipline by providing a directional guide. Sequential warning lights use LED lamp and lens technology and wireless communications technology. Dorman-Dicke Safety Products SynchroGUIDE and Empco-Lite LWCSO are examples of such lights. Only the SynchroGUIDE was tested in this study. The flash rate of the lights is 60 flashes per minute. Each lamp uses two 6V batteries. When the lamps are placed in line, they give the impression of a single light source traveling along the lamps from front to back. The flash or increase in light intensity of each light is synchronized by sensing the location of each light with respect to the other lights. Each lamp has a low output steady light to aid direction indication.

### **The Purpose of Sequential Warning Lights**

In order to minimize traffic impacts due to work zones, departments of transportation (DOTs) have increased off-peak and nighttime work. For example, the Missouri Department of Transportation has a recommendation for off-peak and/or nighttime work when the traffic volumes exceed 75 to 80 percent of the open-lane capacity (MoDOT, 2004).

The increase in nighttime work leads to some potential safety concerns. There is some evidence that nighttime crash characteristics differs from daytime. According to a comprehensive Canadian work zone study (Bushman et al., 2005), crashes under dark conditions have a fatality rate of 2.6 fatalities per 100 crashes while crashes during the day have a rate of 1.8 fatalities per 100 crashes. A U.S. study found that there were more fixed-object crashes and fewer angle and rear-end crashes during the nighttime but no difference in severity (Garber and Zhao, 2002). In discussing the nighttime fixed-object crashes, Garber and Zhao explained that “problems may exist in the lighting conditions at work zones or in the illumination conditions of channelizing devices during nighttime.”

The primary motivation for using sequential warning lights is to improve safety in the work zone by alerting drivers of the upcoming taper and work zone. The British Highway Agency (HA) mentioned that the large number of cone strikes could be due to a driver’s failure to see the taper or to exit the closed lane in sufficient time (HA, 2004).

There are some potential drawbacks to using sequential lights. One is the possibility of photo-sensitive seizure with a wrong flashing rate. Another is the synchronization of driving speeds to sequential warning lights in the tangent section. This might not be a concern for deployments in the short taper area.

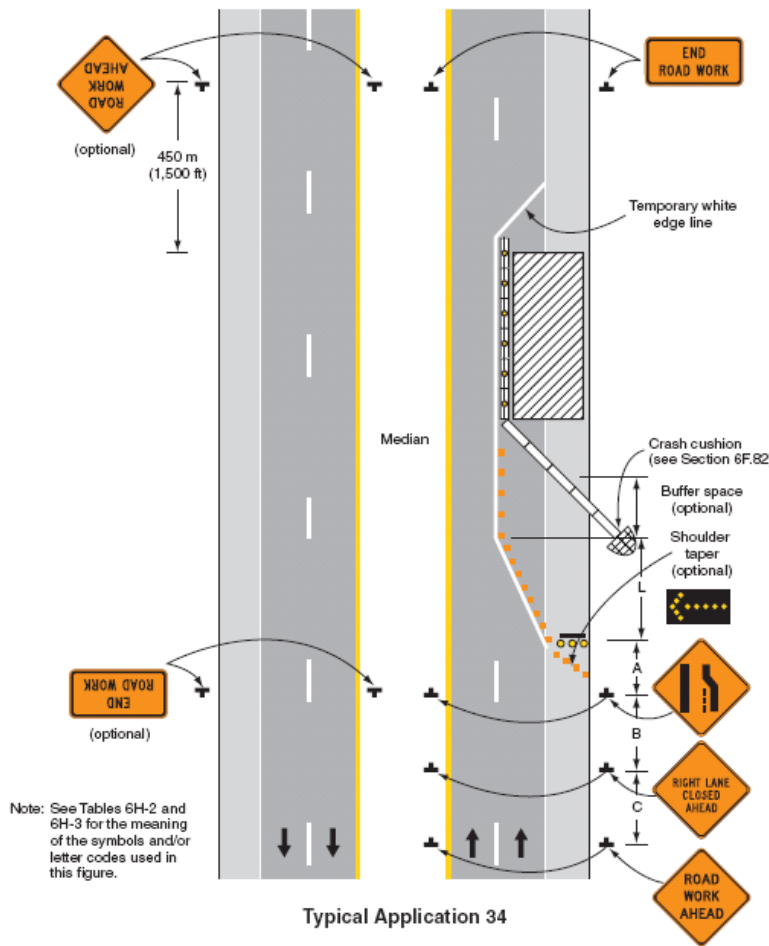
The costs associated with deploying sequential warning lights include labor in deploying the lights, capital cost, and battery replacement cost. Even with the possible drawbacks and costs, sequential barricade lamps were included as option in the latest MUTCD.

### **Technical Background**

Section 6F.59 of the 2003 Manual on Uniform Traffic Control Devices (MUTCD, 2003) specifies that cones equipped with lighting devices can be used for maximizing visibility during nighttime. In Section 6F.78, warning lights are described as portable, powered, yellow, lens-directed and enclosed, and such lights should comply with the ITE Purchase Specifications for Flashing and Steady-Burn Warning Lights (ITE, 2001). The Type C Steady-Burn warning lights may be used during nighttime hours to delineate the edge of the traveled way, and the maximum spacing should be identical to the channelizing device spacing requirements. In Section 6H, several applications are described using the optional warning lights. For example, TA-34 (Lane Closure with Temporary Traffic Barrier) and TA-36 (Lane Shift on Freeway) contain the option for placing Type C Steady-Burn warning lights on channelizing devices for nighttime



lane closures. **FIGURE 1** shows a schematic of TA-34. As shown in **FIGURE 1**, the channelizers shown in orange could all be equipped with sequential lights.



**FIGURE 1 Lane closure with temporary traffic barrier (TA-34) (MUTCD, 2003).**

In Section 6F.63 (Channelizing Devices) of the new MUTCD (FHWA, 2009), the option of using a series of sequential flashing warning lights was introduced as follows:

Option:

12 A series of sequential flashing warning lights may be placed on channelizing devices that form a merging taper in order to increase driver detection and recognition of the merging taper.

Standard:

13 When used, the successive flashing of the sequential warning lights shall occur from the upstream end of the merging taper to the downstream end of the merging taper in order to identify the desired vehicle path. Each warning light in the sequence shall be flashed at a rate of not less than 55 nor more than 75 times per minute.

14 The retroreflective material used on channelizing devices shall have a smooth, sealed outer surface that will display a similar color day or night.

To simplify notation, the term sequential lights will heretofore be used to refer to the sequential flashing warning lights discussed in the Section 6F.63. **FIGURE 2** is an example of such sequential lights. Such lights are battery powered and are NCHRP350 crash compliant. The operating life is dependent on the type of battery and operating conditions but could vary between 230 to 2000 hours.



**FIGURE 2** Sequential warning light (HA, 2005b).

### **Existing Literature and Differences from Previous Studies**

The Texas Transportation Institute (TTI) conducted a study of sequential lights (Finley et al., 2001). The sequential lights were a prototype and were wired. As noted by the evaluators, wired lights could get tangled, so they differ significantly from the wireless lights tested in this study. In addition to controlled sample studies, they also performed field studies on a rural two to one lane work zone and an urban interstate with lanes closed for re-striping work. They measured the occupancy of the closed lane near the taper at: 0 ft, 300 ft, and 1000 ft. They found that such lights may encourage motorists to vacate the closed lane further upstream than normal. However, they did not detect significant lane choice differences at a long term rural test site. The current study measured closed lane occupancy at regular 80 ft intervals instead of at three locations.

The British Highway Agency (HA, 2005b) conducted a trial that involved wireless production-model sequential lights. The trial site was the M42 carriageway which is approximately the equivalent of a U.S. interstate highway. Existing loops were placed 100 m (328 ft) apart and data was collected starting from 1100 m (3609 ft) upstream of the taper. The configuration was a three-lane to two-lane closure.

The main objective of the previous studies was to evaluate the effectiveness of sequential lights. This project builds upon the previous studies and is differentiated by going beyond effectiveness to quantifying the cost-benefit of sequential lights. The previous studies found that the sequential lights were effective. For example, TTI reported that there was a “one-fourth reduction in the number of passenger vehicles and a two-thirds reduction in the number of trucks in the closed lane 1000 ft upstream of the lane

closure.” They also reported that “flashing warning light systems used in the work zone lane closure is perceived positively and is not confusing to the motoring public.” HA reported that the “effect of sequential lamps is seen consistently from a point 500m before the taper, but also has an effect at a point 600m before the taper in half the cases” (HA, 2005a). Since sequential lights are optional and supplementary, agencies need to decide when it is beneficial to deploy them. This project translated measures of effectiveness into quantifiable benefits so that agencies can make decisions concerning the value of deployment.

The wireless production model used for this study differed from the prototype studied in 2001. The sequential lights used by TTI had the limitation of a wired setup and consequently a 900 ft cable length limitation. The evaluators expressed, “the set-up of the system was found to be cumbersome and time-consuming to implement because of the large number of components involved (particularly the use of cables and external junction boxes to interconnect the lights)” (Finely, 2001). To follow up on the previous study, this study included the assessment of the ease of wireless setup by quantifying the required labor effort.

The previous wired setup also caused operational problems. The evaluators mentioned that the system was unable to work properly because “the connections between the junction boxes and the cables tended to lose contact, interrupting the communication signal between lights.” Another objective of this study was to investigate wireless operational issues. Even though wireless operation appeared to be superior, could certain drawbacks exist such as a communications failure between lights?

Another difference from previous studies was the observation of potentially dangerous maneuvers near the beginning of the taper. Such maneuvers include braking near the taper or a sudden merge. This study was also differentiated from the U.K. study, since the U.K. study compared static versus sequential lights. This study involved a comparison of sequential lights on cones with cones with no static lights.

## DATA COLLECTION

The field evaluation of sequential lights was performed on three short-term maintenance work zones on Interstate 70, Missouri. The site geometrics for all the sites were similar involving a right lane closure with the passing lane open (2 to 1 work zone). Field data was collected on two rural work zones on May 17<sup>th</sup> and 18<sup>th</sup>, 2010, and one urban work zone on May 23<sup>rd</sup>. The speed limit on the rural work zone was decreased from 70 mph to 60 mph, while the speed limit on the urban work zone was kept at the normal 60 mph. Thus all three work zones had a speed limit of 60 mph. The details of data collection periods are shown in the **TABLE 1**. **TABLE 1** shows the time periods where data was collected with and without the sequential lights. Road sections in the study sites had minimal horizontal and vertical curves in order to control for geometric factors and to achieve an optimal field-of-view for the data collection equipment. Video data was collected at three different locations near the work zone, and traffic parameters were derived from the video. The locations were at the taper (Near View), just upstream from the taper at the speed radar (Radar View) and approximately 700 feet upstream from the taper (Far View). The video data allowed some automated post-processing of the video and preserved a visual record in case there were anomalies with the data. In addition, the video footage was useful for presenting the results of the study. **FIGURE 3** shows snapshots of sample work zone video footages. **FIGURE 3(a)** shows a set of equally spaced delineators with reflective tops that was used for calibrating distances on the video. The photo also shows the sequential lights mounted on channelizers and the arrow board near the end of the taper. **FIGURE 3(b)** shows the readout of the speed radar at the taper area. **FIGURE 3(c)** shows the closed lane with the adjacent calibration delineators located upstream from the taper.

**TABLE 1 Data Collection Schedule**

	<b>May 17<sup>th</sup></b>	<b>May 18<sup>th</sup></b>	<b>May 23<sup>rd</sup></b>
With lights	10:00PM-11:30PM	11:40PM-1:10AM	9:30PM-11:00PM
Without lights	11:30PM-1:00AM	9:30PM-11:00PM	11:15PM-12:45AM



**FIGURE 3(a) Near View**



**FIGURE 3(b) Radar View**



**FIGURE 3(c) Far View**

**FIGURE 3 Snapshots from video data collection.**

In order to derive traffic and safety parameters, the video was post-processed as follows. First, passenger car parameters were tracked separately from commercial trucks. Second, vehicle speeds with and without sequential lights were recorded. Statistical analysis was performed to assess the significance of the field samples. Third, closed-lane occupancies were collected at selected intervals as an indication of the driver's awareness and action in anticipation of the merge. Fourth, the number of late merges at the taper was tallied. The late merge might be deemed as dangerous maneuvers.

There were three different types of video footage that were processed: Radar, Near Taper and Far Taper. The processing for each type of video is described as follows. The field-of-view of Radar Video contained a view of the taper area and the speed radar display in the lower middle. The information recorded was vehicle speed, vehicle type (passenger car or truck) and the presence of a platoon. Platoon, in this context, meant vehicles following each other within the video field of view. A platoon was determined qualitatively and not based on time headways. The speed had to be recorded manually, since the radar outputted speeds continuously without specifying when it was transitioning between vehicles. Thus it was important to visually and audibly confirm when the radar started to detect the next vehicle. This is especially critical in the case of trucks, since the large physical signature of trucks tend to dominate the radar signature.

The Near Taper Video was processed for conflicts at the taper area. The location of each vehicle was categorized into three categories with respect to the vehicle's transverse location. The three categories were open lane, closed lane and middle. The middle category designates a vehicle over the center line. The number queuing and merging conflicts were noted. A queuing conflict was identified by brake lights from the following vehicle. A merging conflict occurred when a vehicle cut in front of another vehicle.

The Far Taper Video showed the occupancy of the closed lane. The video field-of-view was divided into 80 ft sections that were identified as Zones 1 through 8. The zone where a vehicle moved from the closed to the open lane was noted. The zones were identified using delineators placed upstream from the taper. This calibration of distances in the field was important because delineators appeared to be closer together the further they were located from the camera.

## **DATA ANALYSIS**

### **Radar View**

For the radar view, speed data was analyzed for three field sites. There were two different time periods of data that were collected for each day. These periods were both approximately 90 minutes long and taken consecutively for a combined three-hour time span. As shown in **TABLE 1**, these three-hour time periods took place between approximately 9:30 PM to 1:10 AM. **TABLE 2** is a snippet of the radar speed data. Column 1 shows the five-minute chapter indices that were added for ease of reference. Column 2 shows the speed. Column 3 shows the vehicle type where T stands for commercial trucks and P stands for passenger vehicles. Column 4 indicates the presence and size of a platoon which is determined visually by observing video evidence of vehicles following one another. Only unconstrained vehicle speeds were considered for further analysis, because the speeds of platoon vehicles were constrained by the leading vehicle. The goal was to isolate the effect of the sequential lights on vehicle speed.

**TABLE 2 Example of Radar Data from May 17, 2010**

Chapters (5 min)	Speed (mph)	Vehicle Type (T or P)	Platoon	
1	54	T	1	
	54	T		
	53	T		
	55	T		
	55	P		
	56	T		
	57	P		
	58	T		
	55	P		1
	60	P		
	48	T		
	49	P		
	48	P		
	52	T	1	
	60	P		
	52	P		
	60	P		
	59	T		
	59	T		
	57	T	2	
50	T			
63	P			
64	P	1		
55	P			
55	T			
71	P	1		
49	T			
2	46	T	2	
	41	T	1	

**TABLE 3** presents the descriptive statistics of speeds for total vehicles, passenger cars, trucks, vehicles at rural work zones and vehicles at urban work zones. As explained earlier, only free flow vehicles are included in this table. Thus the Count variable does not include the number of vehicles counted in the platoons. For both with and without lights, **TABLE 3** shows the 85% speeds are around the speed limit for trucks and slightly higher for passenger cars. The speed limit compliance rate is similarly higher for trucks than passenger cars. The standard deviation of speeds and the speed ranges are smaller for trucks than passenger cars. The 85% and mean speeds are both higher at rural work zones as compared to urban work zones. But the standard deviations of speed are higher at urban work zones as compared to rural. **TABLE 3** suggests that a small group of more aggressive drivers skew the overall urban work zone data.



**TABLE 3 Speeds Statistics**  
**3(a) Speed Statistics for Total Vehicles**

	<b>With lights</b>	<b>Without lights</b>
Mean (mph)	55.55	57.76
85 <sup>th</sup> Percentile (mph)	62	63
Standard Deviation (mph)	6.66	5.75
Minimum (mph)	32	34
Maximum (mph)	82	79
Speed Limit Compliance Rate	78.1%	71.4%
Count (veh)	1389	1241

**3(b) Speed Statistics for Passenger Cars**

	<b>With lights</b>	<b>Without lights</b>
Mean (mph)	56.50	58.70
85 <sup>th</sup> Percentile (mph)	63	64
Standard Deviation (mph)	6.73	5.91
Minimum (mph)	32	37
Maximum (mph)	82	79
Speed Limit Compliance Rate	73.1%	65.2%
Count (veh)	900	750

**3(c) Speed Statistics for Trucks**

	<b>With lights</b>	<b>Without lights</b>
Mean (mph)	53.80	56.30
85 <sup>th</sup> Percentile (mph)	60	61
Standard Deviation (mph)	6.15	5.17
Minimum (mph)	32	34
Maximum (mph)	70	71
Speed Limit Compliance Rate	87.3%	80.9%
Count (veh)	489	491

**3(d) Speed Statistics for Rural Work Zones**

	<b>With lights</b>	<b>Without lights</b>
Mean (mph)	57.65	58.43
85 <sup>th</sup> Percentile (mph)	63	63
Standard Deviation (mph)	6.09	5.48
Minimum (mph)	37	35
Maximum (mph)	82	79
Speed Limit Compliance Rate	69.0%	68.3%
Count (veh)	749	861

**3(e) Speed Statistics for Urban Work Zone**

	With lights	Without lights
Mean (mph)	53.09	56.24
85 <sup>th</sup> Percentile (mph)	60	62
Standard Deviation (mph)	6.45	6.06
Minimum (mph)	32	34
Maximum (mph)	69	75
Speed Limit Compliance Rate	88.8%	78.4%
Count (veh)	640	380

A t-test is a common statistical test for determining if sample means from different samples are statistically different. T-tests were performed on the “with lights” and “without lights” speed data. The test statistic is given by,

$$\frac{(\bar{X}_1 - \bar{X}_2) - 0}{\sqrt{S_1^2 / n_1 + S_2^2 / n_2}}$$

where  $\bar{X}_1$ ,  $\bar{X}_2$  are the sample means with and without sequential lights and  $S_1^2$ ,  $S_2^2$  are the sample variances of with and without sequential lights, and  $n_1$  and  $n_2$  are the sample sizes (Milton and Arnold, 1995).

The t-test results are shown in **TABLE 4**. All the null hypothesis rejections indicate there is a significant difference in the mean speeds with and without sequential lights for all analysis categories (all vehicles, passenger cars, trucks, vehicles at rural work zones and vehicles at urban work zones.) The p-values were all close to a value of 0. As shown in **TABLE 4**, Sequential lights resulted in a statistically significant mean speed reduction of 2.5 mph for all vehicles, 2.2 mph for passenger cars and 2.5 mph for trucks. Mean speeds decreased by 0.8 mph and 3.1 mph for the vehicles in rural work zones and urban work zones, respectively due to the installation of sequential lights. The greater effect on trucks was expected as trucks have more limited performance characteristics, and truck drivers are more regulated and receive more training than non-commercial drivers.

**TABLE 4 T-Test Results for Mean Speeds**

	Hypothesis	Mean w/ lights	Mean w/o lights	Change	P-value	Reject null hypothesis?
All vehicles	$H_0: \mu_{with} = \mu_{without}$	55.55	57.76	-2.21	0.000	Yes
	$H_1: \mu_{with} < \mu_{without}$					
Passenger cars	$H_0: \mu_{with} = \mu_{without}$	56.50	58.70	-2.2	0.000	Yes
	$H_1: \mu_{with} < \mu_{without}$					
Trucks	$H_0: \mu_{with} = \mu_{without}$	53.80	56.30	-2.5	0.000	Yes
	$H_1: \mu_{with} < \mu_{without}$					
Rural WZ	$H_0: \mu_{with} = \mu_{without}$	57.65	58.43	-0.78	0.004	Yes
	$H_1: \mu_{with} < \mu_{without}$					
Urban WZ	$H_0: \mu_{with} = \mu_{without}$	53.09	56.24	-3.15	0.000	Yes
	$H_1: \mu_{with} < \mu_{without}$					

Key:  $\mu_{with}$  is the mean speed of vehicles at work zones with sequential warning lights

$\mu_{without}$  is the mean speed of vehicles at work zones without sequential warning lights

Despite some vigorous debate over the years, it is generally accepted that vehicle speeds are correlated to crash severities (TRB, 1998). The 85% speed was examined more carefully as it is commonly used for establishing the speed limit. As shown in **TABLE 5**, the 85% speeds with sequential lights were lower than those without sequential lights for all vehicles, passenger cars and trucks. The significance of the difference in 85% speeds was tested by using a standard normal Z test. The test statistic is

$$\frac{(X_{(n0.85)+1} - Y_{(n0.85)+1}) - 0}{1.53\sqrt{S_X^2/n_X + S_Y^2/n_Y}}$$

where  $X_{(n0.85)+1}$  is the sample 85% speed with sequential lights,  $Y_{(n0.85)+1}$  is the sample 85% speed without sequential lights, and  $S_X^2$ ,  $S_Y^2$  are the sample variances of with and without sequential lights, and  $n_1$  and  $n_2$  are the sample sizes (Crammer, 1946). **TABLE 5** shows the differences in the 85% speed was statistically significant. **TABLE 5** shows there is no difference in the 85% speed in rural work zones while there is a statistically significant difference in the urban work zone.

**TABLE 5 Standard Normal Z Test Results for 85<sup>th</sup> Percentile Speed**

	Hypothesis	85% speed with lights	85% speed w/o lights	Change	P- value	Reject null hypothesis?
All vehicles	$H_0 : (\xi_{0.85})_{with} = (\xi_{0.85})_{without}$	62	63	-1	0.003	Yes
	$H_1 : (\xi_{0.85})_{with} < (\xi_{0.85})_{without}$					
Passenger cars	$H_0 : (\xi_{0.85})_{with} = (\xi_{0.85})_{without}$	63	64	-1	0.017	Yes
	$H_1 : (\xi_{0.85})_{with} < (\xi_{0.85})_{without}$					
Trucks	$H_0 : (\xi_{0.85})_{with} = (\xi_{0.85})_{without}$	60	61	-1	0.035	Yes
	$H_1 : (\xi_{0.85})_{with} < (\xi_{0.85})_{without}$					
Rural WZ	$H_0 : (\xi_{0.85})_{with} = (\xi_{0.85})_{without}$	63	63	0	0.500	No
	$H_1 : (\xi_{0.85})_{with} < (\xi_{0.85})_{without}$					
Urban WZ	$H_0 : (\xi_{0.85})_{with} = (\xi_{0.85})_{without}$	60	62	-2	0.001	Yes
	$H_1 : (\xi_{0.85})_{with} < (\xi_{0.85})_{without}$					

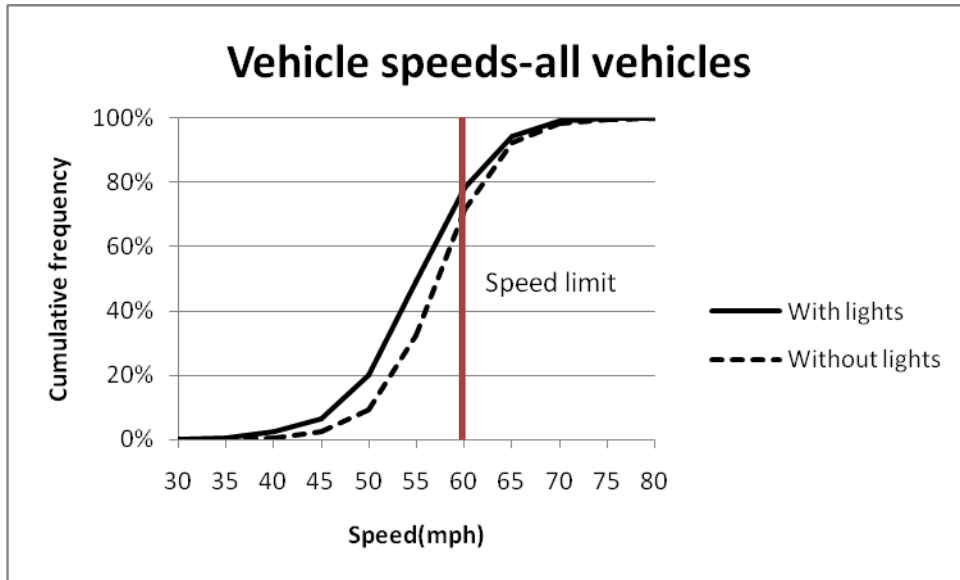
Key:  $(\xi_{0.85})_{with}$  is the 85<sup>th</sup> percentile speed with sequential warning lights

$(\xi_{0.85})_{without}$  is the 85<sup>th</sup> percentile speed without sequential warning lights

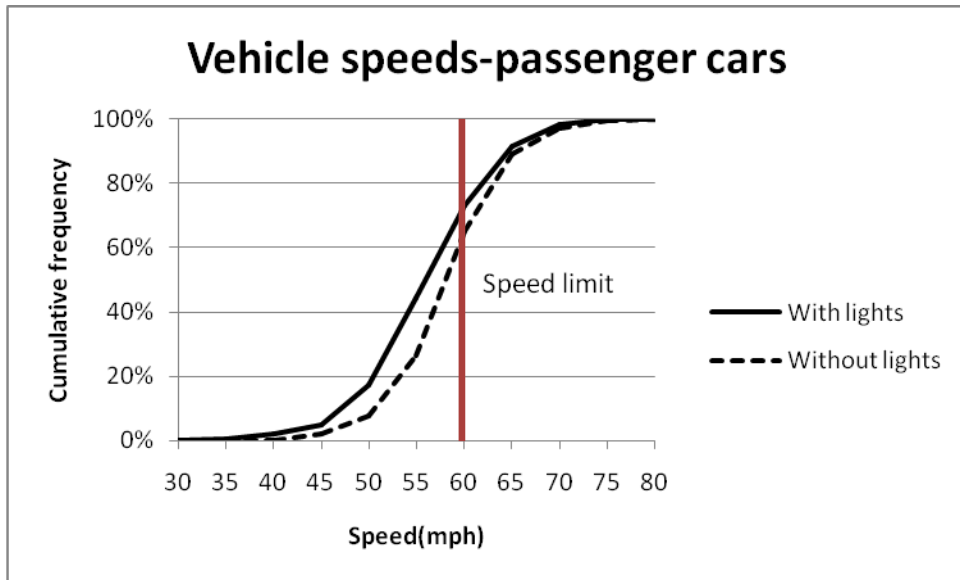
In **FIGURE 4**, cumulative speed distributions of free flowing vehicles with sequential lights and without sequential lights are shown and compared. The speed limit of 60 mph is shown as a red vertical line. Whether or not this line falls above or below the 85% speed has implications for speed compliance and safety. With sequential lights, the distribution curves of total vehicles, passenger cars, trucks, vehicles at rural work zones and vehicles at urban work zones were all shifted to the left, indicating a decrease in vehicle speeds. The results of the comparison of vehicle speeds at rural work zones show only vehicle speeds below 60 mph were reduced by sequential lights as shown in **FIGURE 4(d)**. All of the other comparisons indicate that sequential lights decrease the speeds of all vehicles in the study: passenger cars, trucks and vehicles at urban work zones in all speed ranges. To determine if the speed distributions differences (with and without lights) in the five data sets shown in **FIGURE 4** are statistically significant, two commonly used statistical tests, Mann-Whitney U test and Kolmogorov-Smirnov test (Conover, 1980), were applied. The results are displayed in **TABLE 6**. In all five data sets, the cumulative speed distributions with sequential lights were significantly different from those without sequential lights.

**TABLE 6 Results of Mann-Whitney U Test and K-S Test**

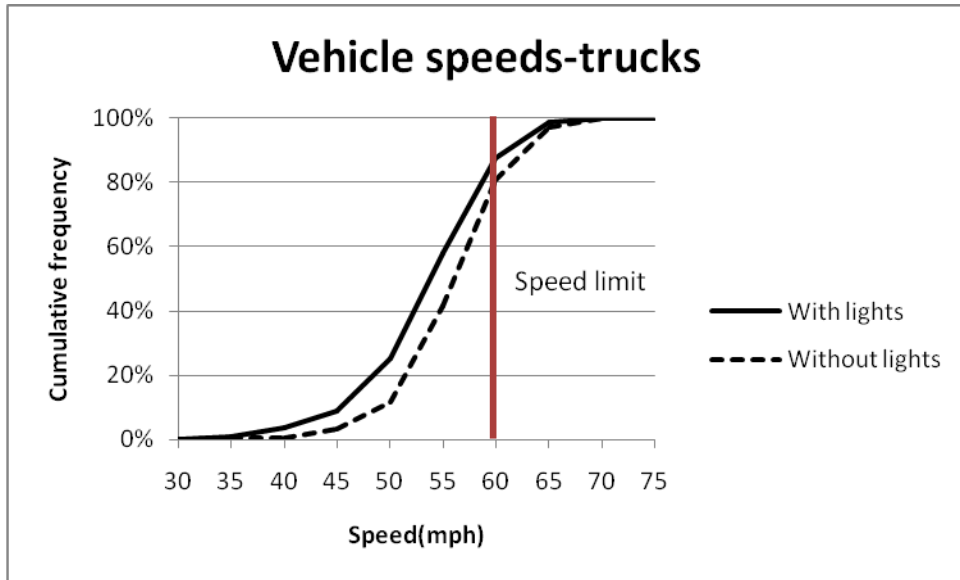
	P-value: Mann-Whitney	P-value: K-S	Statistical Significant?
All vehicles	0.000	0.000	Yes
Passenger cars	0.000	0.000	Yes
Trucks	0.000	0.000	Yes
Rural WZ	0.000	0.000	Yes
Urban WZ	0.000	0.000	Yes



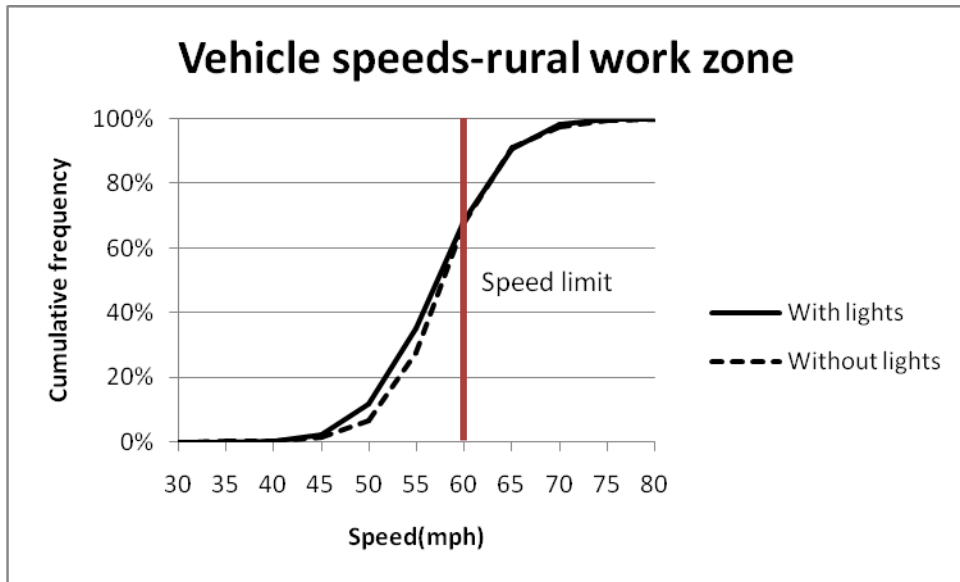
4(a) Total vehicles



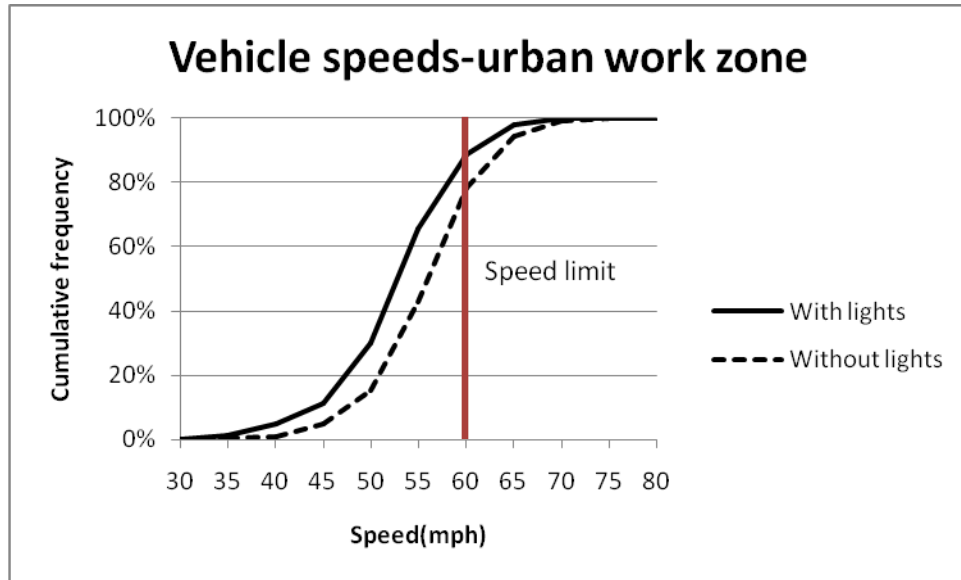
4(b) Passenger cars



4(c) Trucks



4(d) Rural work zone



4(e) Urban work zone

**FIGURE 4 Cumulative speed distributions comparisons: with and without sequential lights.**

The F-test is a common statistical test for comparing variability between two samples by analyzing the ratio of variances from the samples. The standard deviations of vehicle speeds were analyzed statistically using the F-test. The test statistic of F-test is specified as,

$$\frac{S_1^2}{S_2^2}$$

where  $S_1^2$  and  $S_2^2$  are the sample variances of two populations to be compared (Milton and Arnold, 1995).

The results of the test are shown in **TABLE 7**. All null hypotheses were rejected showing that there were statistically significant differences in the standard deviations of vehicle speed for all categories of data. Thus, sequential lights slightly increased standard deviations by 0.91 mph on all vehicles, 0.82 mph on passenger cars, 0.98 mph on trucks, 0.61 mph on vehicles at rural work zone and 0.39 mph on vehicles at urban work zone.

**TABLE 7 F-Test Results for Speed Variances**

	Hypothesis	Std. dev. with light	Std. dev. w/o lights	Change	P-value	Reject null hypothesis?
All vehicles	$H_0 : \sigma_{with} = \sigma_{without}$	6.66	5.75	0.91	0.000	Yes
	$H_1 : \sigma_{with} > \sigma_{without}$					
Passenger cars	$H_0 : \sigma_{with} = \sigma_{without}$	6.73	5.91	0.82	0.000	Yes
	$H_1 : \sigma_{with} > \sigma_{without}$					
Trucks	$H_0 : \sigma_{with} = \sigma_{without}$	6.15	5.17	0.98	0.000	Yes
	$H_1 : \sigma_{with} > \sigma_{without}$					
Rural WZ	$H_0 : \sigma_{with} = \sigma_{without}$	6.09	5.48	0.61	0.001	Yes
	$H_1 : \sigma_{with} > \sigma_{without}$					
Urban WZ	$H_0 : \sigma_{with} = \sigma_{without}$	6.45	6.06	0.39	0.089	Yes
	$H_1 : \sigma_{with} > \sigma_{without}$					

Key:  $\sigma_{with}$  is the standard deviation of vehicle speed with sequential warning lights

$\sigma_{without}$  is the standard deviation of vehicle speed without sequential warning lights

In addition, drivers' speed limit compliance rate with and without sequential lights were examined. A standard normal Z test was used to test the significance of the difference in compliance rate. The two sample Z test statistic is,

$$\frac{(\hat{p}_1 - \hat{p}_2) - 0}{\sqrt{\hat{p}_1(1 - \hat{p}_1)/n_1 + \hat{p}_2(1 - \hat{p}_2)/n_2}}$$

Where  $\hat{p}_1$  and  $\hat{p}_2$  are the sample proportions of two populations, and  $n_1$  and  $n_2$  are the two sample sizes (Milton and Arnold, 1995).

The speed limit compliance Z test results are presented in **TABLE 8**. The test shows no significant difference in passenger car compliance rate between with and without sequential lights. However, all other null hypotheses were rejected which means sequential lights had a statistically significant effect in increasing driver compliance with posted work zone speed limit.



**TABLE 8 Standard Normal Z Test Results for Compliance Rate**

	Hypothesis	Percentage with light	Percentage w/o lights	Change	P-value	Reject null hypothesis?
All vehicles	$H_0: p_{with} = p_{without}$	78.1%	71.4%	-6.7%	0.000	Yes
	$H_1: p_{with} > p_{without}$					
Passenger cars	$H_0: p_{with} = p_{without}$	73.1%	65.2%	-7.9%	0.000	Yes
	$H_1: p_{with} > p_{without}$					
Trucks	$H_0: p_{with} = p_{without}$	87.3%	80.9%	-6.4%	0.003	Yes
	$H_1: p_{with} > p_{without}$					
Rural WZ	$H_0: p_{with} = p_{without}$	69.0%	68.3%	-0.7%	0.381	No
	$H_1: p_{with} > p_{without}$					
Urban WZ	$H_0: p_{with} = p_{without}$	88.8%	78.4%	-10.4%	0.000	Yes
	$H_1: p_{with} > p_{without}$					

Key:  $p_{with}$  is the drivers' speed limit compliance percentage with sequential warning lights

$p_{without}$  is the drivers' speed limit compliance percentage without sequential warning lights

### Near Taper Conflict View

The positions of vehicles at the taper were recorded for all three field sites. The vehicles were categorized as being in the open lane, closed lane, or in the process of moving from the closed to the open lane (middle). An example of near taper is presented in **TABLE 9**. Column 1 shows the five-minute chapter indices that were added for ease of reference. Column 2, Column 3 and Column 4 show the vehicle type in the open lane, the middle and closed lane respectively. T stands for commercial trucks and P stands for passenger vehicles. Column 5 indicates the presence of a platoon. Vehicles in the middle and closed lane near the taper were construed as late merges. **TABLE 10** displays basic vehicle counts for the near taper view.

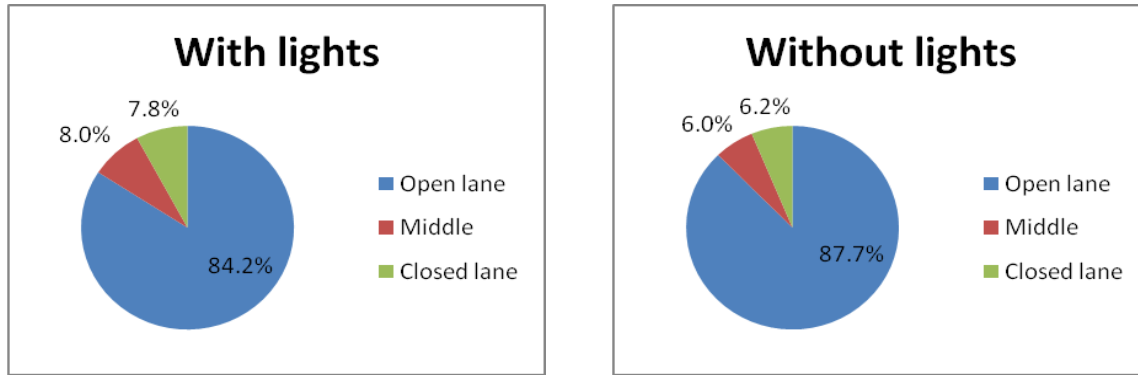
**TABLE 9 Example of Near Taper Data from May 17, 2010**

Chapters (5 min)	Open Lane (T or P)	Middle (T or P)	Closed Lane (T or P)	Queuing
8			T	2
	P			1
	P			
	T			1
	T			1
	T			
	T			1
	P			1
	T			
	P			
	T			
	T			1
	P			
	T			

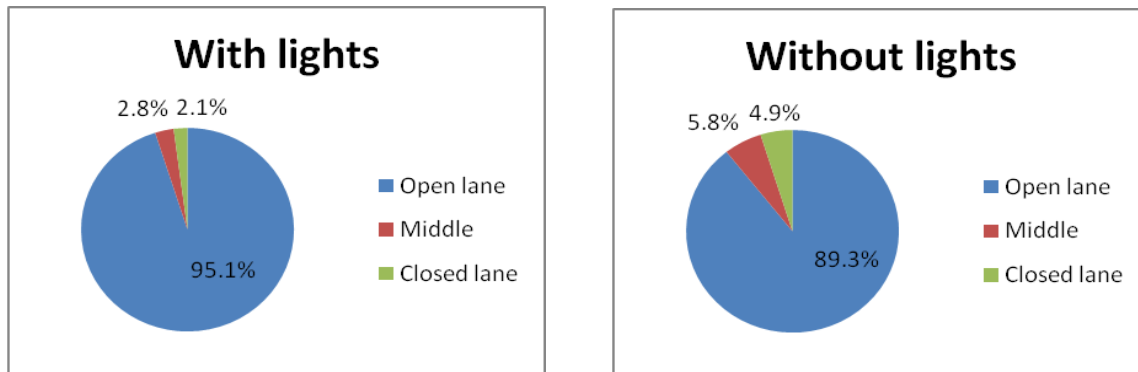
**TABLE 10 Vehicle Counts in Near Taper Test**

	(5/17) with lights	(5/17) w/o lights	(5/18) w/o lights	(5/18) with lights	(5/23) with lights	(5/23) w/o lights
Total Vehicles	543	368	854	487	1206	508
Vehicles in Platoon	160	86	498	196	602	153
Analysis Vehicles	383	282	356	291	604	355

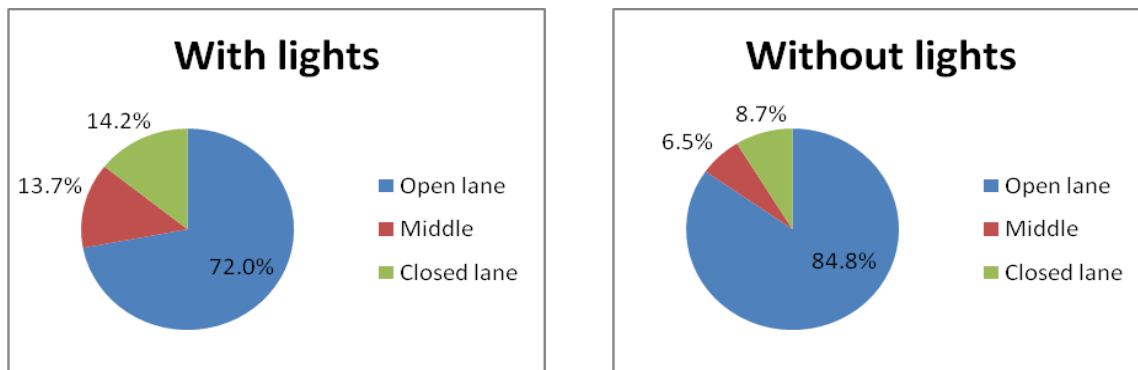
The percentage of vehicle occupancy in open, middle, and closed lanes near the taper are presented in **FIGURE 5 (a)**. When the with and without sequential lights are compared, it was found that 7.8% of vehicles were in the closed lane with sequential lights in contrast to 6.2 % without sequential lights, and 8.0% of vehicles were in the middle with sequential lights in contrast to 6.0% without sequential lights. It appeared that sequential lights had a negative effect, because there were a higher percentage of vehicles in the closed or middle lane near the taper. The vehicle occupancies were further investigated separately for rural and urban work zone datasets. The results are shown in Figures 5 (b) and 5(c). For rural work zone data, the results show that both the percentage of vehicles in the closed lane and in the middle decreased by 2.8% and 3.0% with the deployment of sequential lights. On the other hand, for urban work zone data, both percentage of vehicles in the closed lane and middle increased by 5.5% and 7.2% with the deployment of sequential lights. One possible reason for the increase in the late mergers with sequential lights in urban work zone was that a small portion of aggressive drivers waited longer to merge as they were more able to estimate the location of the taper illuminated by sequential lights. Also, in general, urban areas have more lighting near the highway from other businesses as compared to rural areas aiding the visibility of taper during night time. Separate analysis for passenger cars and trucks in closed lane and middle was not conducted as there were few trucks in the closed lane near the taper.



5(a) Overall three-day data



5(b) Rural work zone data



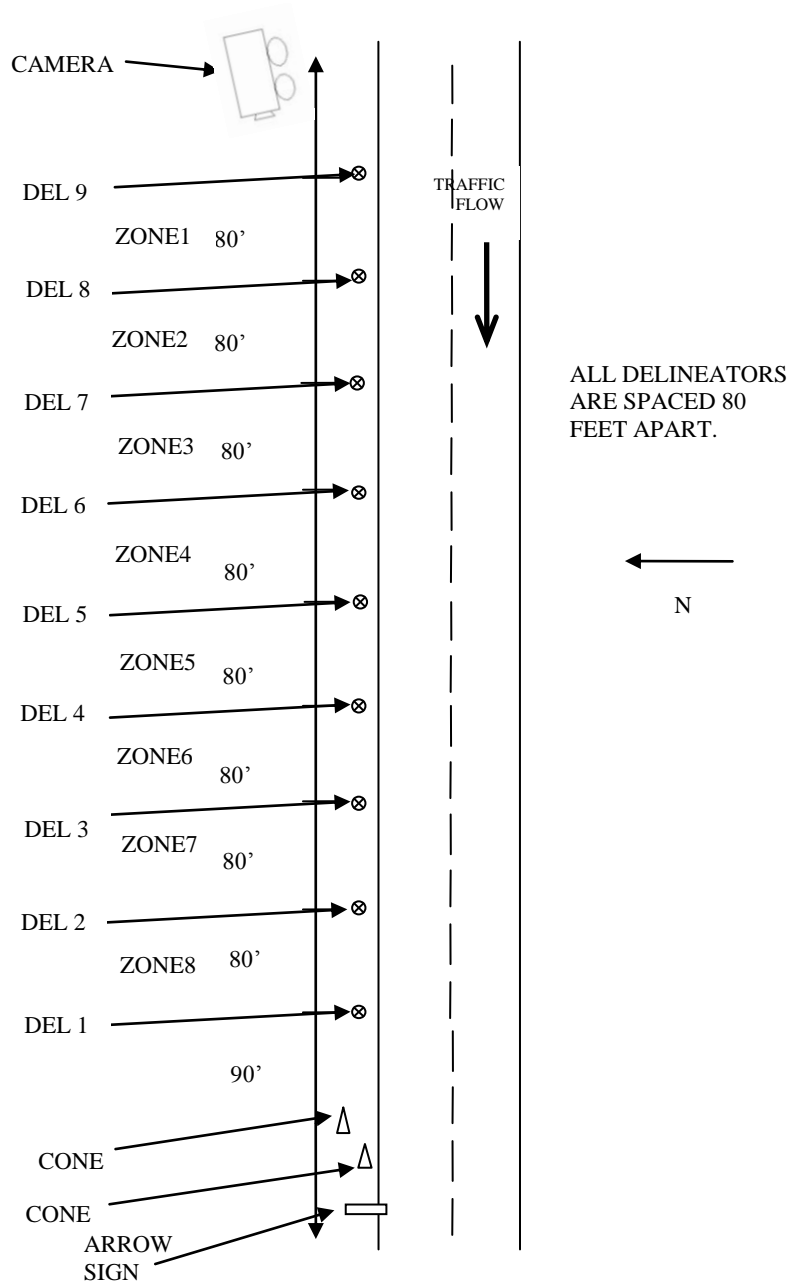
5(c) Urban work zone data

FIGURE 5 Frequency of vehicles in the open lane, middle, and closed lane near the taper.

### Far Taper View

For this view, the area upstream from the taper was divided into eight zones using delineators placed on the shoulder as shown in **FIGURE 6**. Vehicles were then classified into each of these zones based on where they merged into the open lane. Zone 8 is closest to the work zone taper being approximately 90 feet from the taper. Each zone is 80 feet long. Vehicles merging in the early zones, e.g. Zone 1, were safer because they were farther away from the lane closure. **TABLE 11** shows an example of the analysis performed on one of the field sites. Column 1 shows the five-minute chapter indices that were added for

ease of reference. Column 2 shows the vehicle type in the open lane. Columns 3 to 10 show the vehicle type and where the vehicle merged from the closed to the open lane. The vehicle counts are shown in **TABLE 12**.



**FIGURE 6** Layout of delineator setting in the field.

**TABLE 11 Example of Far Taper Test Data from May 17, 2010**

Chapters (5 min)	Open Lane T or P	Z1 T or P	Z2 T or P	Z3 T or P	Z4 T or P	Z5 T or P	Z6 T or P	Z7 T or P	Z8 T or P
1	T								
	T								
	P								
	T								
	P								
	P								
	T								
	T								
	P								
	T								
	T								
	P								
	P								
	P								
	P								
								P	
	P								
	P								
									T
	P								
	P								
									P

**TABLE 12 Vehicle Counts of Far Taper Test**

	Total Vehicles	Passenger Cars	Trucks	Rural WZ	Urban WZ
With lights	2417	1723	697	1198	1219
Without lights	1699	1101	598	1189	510

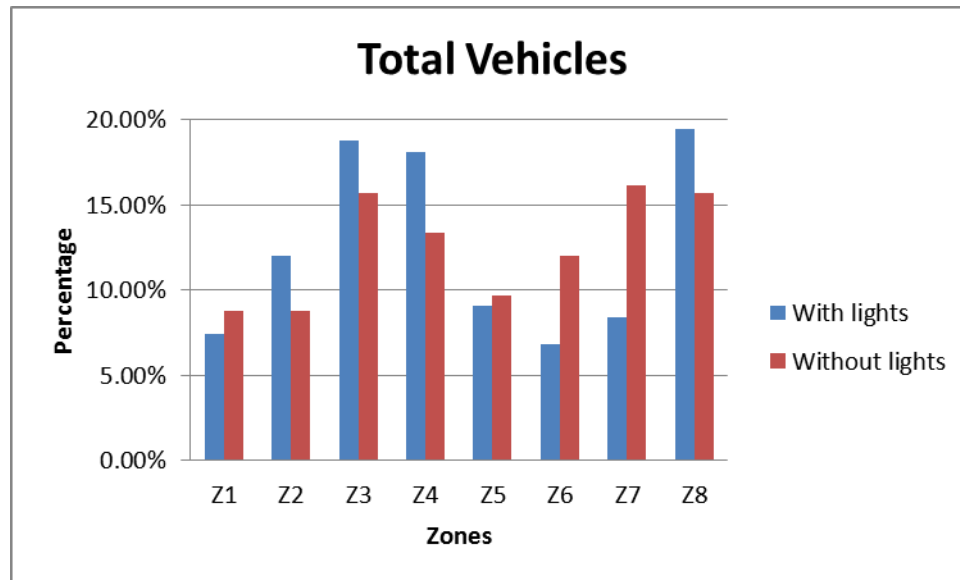
**FIGURE 7** shows the percentage of vehicles merging into the open lane at different zones with and without sequential lights. Total vehicles, passenger cars, trucks, rural work zone and urban work zone were analyzed separately. After deploying sequential lights, as shown in **FIGURE 7(a)** and **7(b)**, the percentage of total vehicles and passenger vehicles merging into the open lane shifted away from the taper. Vehicles merged earlier in anticipation of the lane closure in the with lights scenario. Thus there were fewer vehicles in Zones 5-8 and more vehicles in Zone 1-4. The only exception was an actual increase in the percentage of vehicles merging in Zone 8, the zone closest to the taper. This exception further supports our finding from the near taper conflict analysis that a small portion of aggressive drivers delayed their merge until they reached the taper because of the enhanced visibility of sequential lights. The percentage of passenger cars merging in the first five zones increased from 58.62% to 65.36% (or 6.74% increase) when sequential lights were deployed.

As shown in **FIGURE 7(c)**, the percentage of trucks merging in the first five zones increased from 46.51% to 65.52% (or 19% increase) when sequential lights were deployed. While the percentage of trucks merging in the last three zones decreased from 53.49% to 34.48%. Sequential lights had a more pronounced effect on trucks than passenger cars, because there was a more significant shift to earlier zones for trucks, and there was a decrease in merging in Zone 8. This finding is intuitive as trucks are

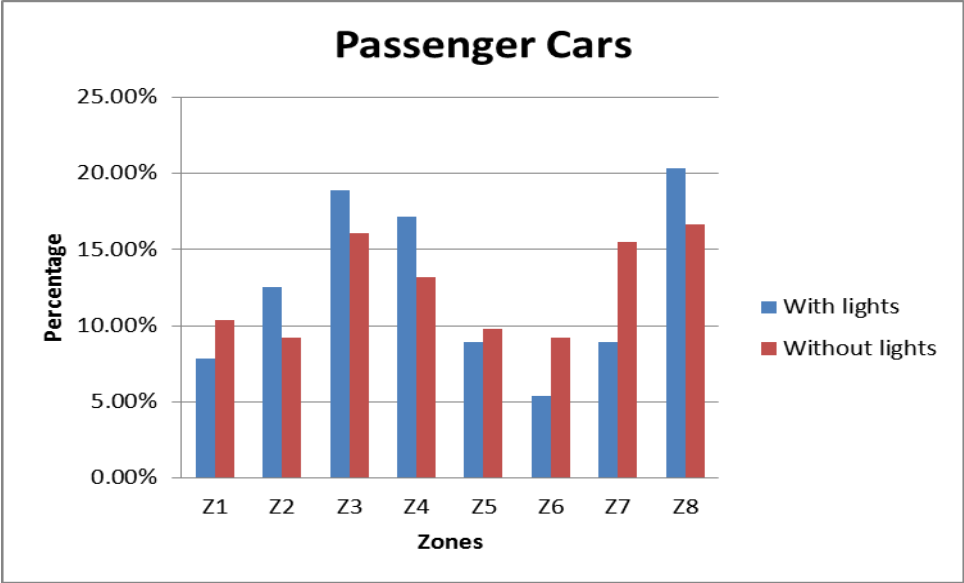
more limited in performance than passenger cars, and truck drivers are more regulated and receive more training.

In the rural work zone scenario, as shown in **FIGURE 7(d)**, the percentage of vehicles merging within the first five zones increased from 34.91% to 45.61% when sequential lights were deployed (or 10.71%). In the urban work zone scenario as shown in **FIGURE 7(e)**, the percentage of vehicle merging within the first five zones decreased from 76.58% to 69.84% (or 6.74% decrease). Thus, it appeared that there were somewhat contradictory results between the rural and urban settings. This inconsistency can partially be attributed to the presence of higher traffic volumes when sequential lights were deployed. More traffic meant fewer gaps in the open lane that led to some drivers delaying their merge closer to the taper. As shown in **TABLE 12**, the dataset with sequential lights consisted of 1219 vehicles which was more than two times the dataset without sequential lights (510 vehicles).

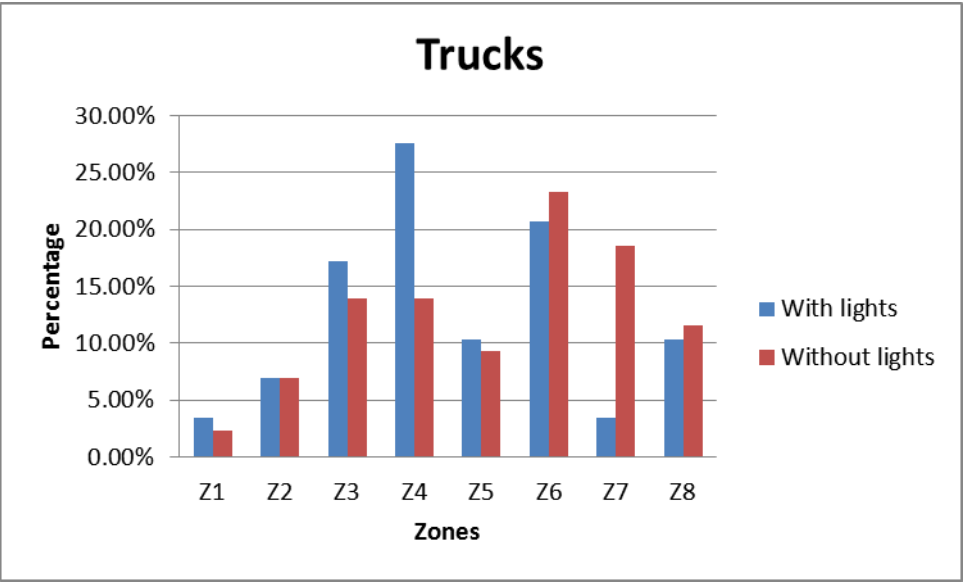
There is some evidence that vehicles have merged earlier with sequential lights even upstream of the eight zones. The strongest effects are present in the rural and truck cases. In rural work zones, 4.76% of the total traffic merged within the eight zones with sequential lights, and 8.92% of the total traffic merged within the eight zones without sequential lights. With trucks only, 4.16% of the total truck traffic merged within the eight zones with sequential lights, and 7.19% of the total truck traffic merged within the eight zones without sequential lights.



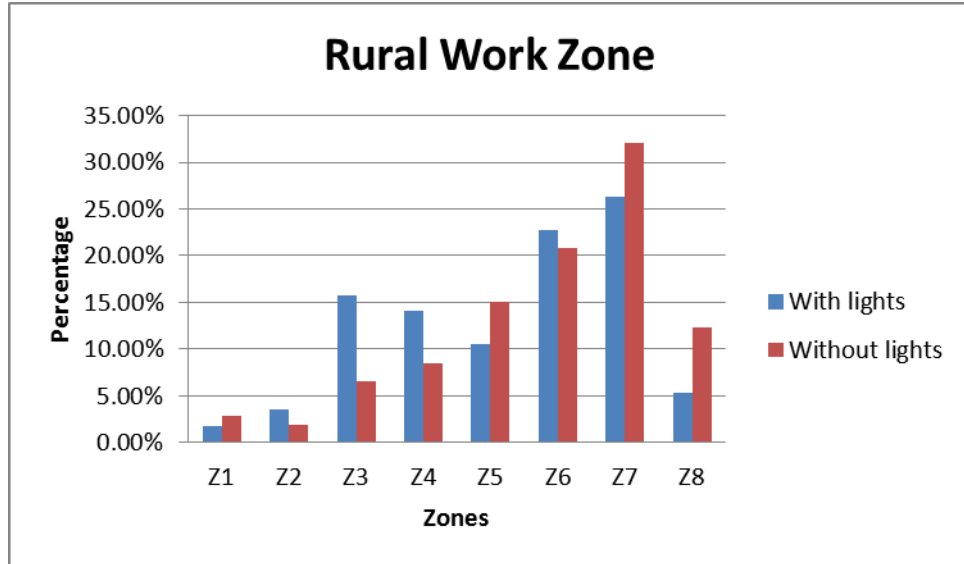
7(a) Total vehicles



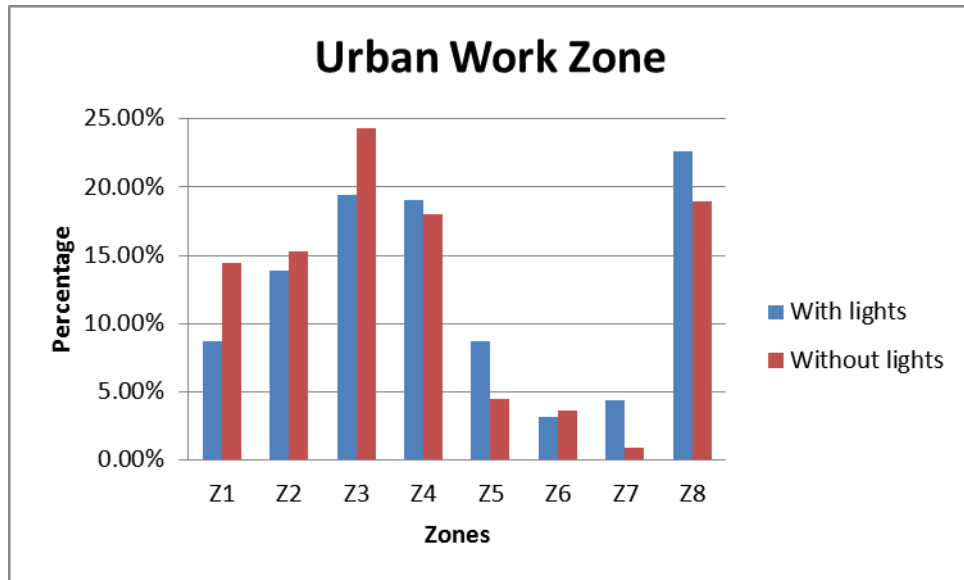
7(b) Passenger cars



7(c) Trucks



7(d) Rural work zone



7(e) Urban work zone

**FIGURE 7 Percentage of vehicles merging at different zones.**

In addition to analyzing the effects of sequential lights on merge percentage at different zones, the average merge distance from the taper was calculated for the vehicles that merged within the eight zones. The average merge distance from taper ( $\bar{L}$  feet) was estimated by dividing the summation of the product of the distance from the taper to the center of each zone ( $l_i$ ) and the number of vehicles merging into the open lane in each zone ( $n_i$ ) by the total number of merging vehicles ( $N$ ). It is specified by

$$\bar{L} = \frac{\sum_{i=1}^8 l_i \cdot n_i}{N} . \quad (1)$$



The average merge distances are shown in **TABLE 13**. With sequential lights, the average merge distance of all vehicles, passenger cars, trucks and vehicles at rural work zone were all longer than without sequential lights. The average merge distance from taper of all vehicles with sequential lights was 20 feet longer than without sequential lights. The average merge distance of passenger cars and trucks with sequential lights are 13 and 49 feet longer than without sequential lights. At rural work zones, the average merge distance was lengthened 44 feet by the use of sequential lights. However, at the urban work zone, the average merge distance with sequential lights was 42 feet shorter than without sequential lights. This is consistent with the findings of the close lane occupancies as shown in **FIGURE 7(e)**. Again, the anomaly of the urban work zone could have been the result of a subset of more aggressive drivers wanting to overtake near the taper.

**TABLE 13 The Average Merge Distances From Taper**

	Average merge distance from taper (ft)				
	All vehicles	Passenger Cars	Trucks	Rural Work Zones	Urban Work Zones
With lights	402	402	400	348	414
Without lights	382	389	351	304	456

## ECONOMIC ANALYSIS

The final aspect of this study was the economic analysis of the benefits and costs of deploying sequential lights. The tangible safety benefits were estimated and valued economically. Such benefits were computed from the potential reductions in crashes at nighttime work zones in Missouri. The deployment costs included the cost of sequential warning lights and batteries, and labor costs for installation and removal.

The benefit-cost analysis involved the following assumptions. Only fatal and injury crashes were considered in computing the benefits. Injuries included both disabling and minor injuries. In contrast, the costs of property-damage-only (PDO) crashes were considerably less significant. This case study only used work zone crash data from freeways and major highways. Thus no work zones on interrupted flow facilities were considered. Also, only Missouri data was used for this study. But the results from this study could be adapted to other states by using the appropriate crash data for the other states.

### Total Benefits

Total benefits ( $B_{Total}$  dollars per year) from sequential lights were computed by taking the difference between the total costs of crashes without sequential warning lights ( $C_{Total,Without}^{Crash}$  dollars per year) and the total costs of crashes with ( $C_{Total,With}^{Crash}$  dollars per year) sequential lights. Thus, total benefits were specified as

$$B_{Total} = C_{Total,Without}^{Crash} - C_{Total,With}^{Crash} \quad (2)$$

The total costs of crashes without sequential warning lights,  $C_{Total,Without}^{Crash}$ , was obtained using historical crash data. This total cost of crashes was composed of the total costs of fatal crashes ( $C_{Total,Without}^{Fatal}$  dollars per year) and the total costs of injury crashes ( $C_{Total,Without}^{Injury}$  dollars per year), i.e.

$$C_{Total,Without}^{Crash} = C_{Total,Without}^{Fatal} + C_{Total,Without}^{Injury} = N_{Without}^{Fatal} \times C_{Without}^{Fatal} + N_{Without}^{Injury} \times C_{Without}^{Injury}, \quad (3)$$

where  $N_{Without}^{Fatal}$  ( $N_{Without}^{Injury}$ ) is number of fatal (injury) crashes per year and  $C_{Without}^{Fatal}$  ( $C_{Without}^{Injury}$ ) is the average cost per fatal (injury) crash.

Since sequential lights were a relatively new technology, there was no significant crash data associated with their deployment, thus crash regression models were used to estimate the crash benefits of sequential lights. The use of crash regression models is an accepted method that is used in publications such as the Redbook (AASHTO, 2003). Two regression models were considered in the study. One was the Power Model, originally derived by Nilsson (2004). This model expressed the quantitative relationship between crash and speed and is given by

$$\frac{n_1}{n_0} = \left( \frac{V_1}{V_0} \right)^\alpha, \quad (4)$$

where  $n_1$  was the number of fatal or injury crashes at mean speed  $V_1$ ,  $n_0$  was the number of fatal or injury crashes at mean speed  $V_0$ , and  $\alpha = 4$  for fatal crashes and  $\alpha = 2$  for injury crashes. Another model was one proposed by Garber and Ehrhart (2000) that expressed the mathematical relationship between crash rate and several factors, including mean speed, speed variance, and flow. For freeways with speed standard deviation ranging from 8 km/h to 18 km/h, mean speed ranging from 90 km/h (55 mph) to 98 km/h (60 mph) and flow ranging from 200 veh/h/lane to 1800 veh/h/lane, the model form was

$$\begin{aligned} Crashrate = & (0.355) - (1.591 \times 10^{-3}) \times (SD^2) + (8.651 \times 10^{-7}) \times (SD^4) - (2.071 \times 10^{-8}) \times \\ & (FPL^2) - (1.256 \times 10^{-10}) \times (SD^2) \times (FPL^2) + (8.527 \times 10^{-15}) \times (FPL^4) - (6.509 \times 10^{-5}) \times \\ & (MEAN^2) + (1.725 \times 10^{-7}) \times (SD^2) \times (MEAN^2) + (3.143 \times 10^{-12}) \times (FPL^2) \times (MEAN^2) + \\ & (2.875 \times 10^{-9}) \times (MEAN^4) \end{aligned} \quad (5)$$

where  $Crashrate$  was in terms of the number of crashes per hour per km per lane,  $SD$  was the standard deviation of speed (km/h),  $FPL$  was the flow per lane (veh/h/lane) and  $MEAN$  was the mean speed (km/h).

Both models were similar in expressing the non-linear relationship between crash rate and speed. However, Nilsson's Power Model treated fatal and injury crashes separately. In the Garber-Ehrhart model, flow and standard deviation of speed were included, but the model was developed based on speed data collected at 55 mph speed limit locations, not the 60 mph speed limit at the work zones investigated in this study. Also, crash rate in the Garber-Ehrhart model included all type of severity crashes such as fatal, injury, and property damage only (PDO). In this study, only fatal crash and injury crash were investigated for the economic analysis. In addition, the Power Model was widely used and was accepted by the European Commission (EC, 1999) as a method to express the relationship between speed and crashes. Hence, Nilsson' Power Model was a better fit for this study.

According to the Power Model, the predicted ratio of the number of crashes with installation of sequential warning lights to the number of crashes without was given by

$$R_{Fatal} = \left( \frac{V_{With}}{V_{Without}} \right)^4 \text{ and } R_{Injury} = \left( \frac{V_{With}}{V_{Without}} \right)^2, \quad (6)$$

where  $R_{Fatal}$  was the ratio for fatal crashes,  $R_{Injury}$  was the ratio for injury crashes,  $V_{With}$  was the mean speed with sequential warning lights (mph), and  $V_{Without}$  was the mean speed without sequential warning lights (mph).

The total costs of crashes with the installation of sequential warning lights ( $C_{Total,With}$ ) were expressed as

$$C_{Total,With}^{Crash} = N_{Without}^{Fatal} \times C_{Without}^{Fatal} \times R_{Fatal} + N_{Without}^{Injury} \times C_{Without}^{Injury} \times R_{Injury}. \quad (7)$$

By substituting (3) and (7) to (2), the total benefits were computed as

$$B_{Total} = N_{Without}^{Fatal} \times C_{Without}^{Fatal} \times (1 - R_{Fatal}) + N_{Without}^{Injury} \times C_{Without}^{Injury} \times (1 - R_{Injury}). \quad (8)$$

**TABLE 14** shows the fatal and injury ratios computed using speeds measured in the three work zone sites. **TABLE 15** presents the nighttime work zone crash history on US freeways and major interstates in Missouri for the last five years. Only nighttime work zone crashes were considered for this analysis because sequential lights could have the most impact at nighttime. **TABLE 16** shows the user costs of crashes from the Redbook (AASHTO, 2003). The total costs of fatal crashes without installation of sequential warning lights  $C_{Total,Without}^{Fatal}$  were estimated to be 4.4 fatal crashes/year  $\times$  \$3.72 million per fatal crash, or \$16.37 million in 2000 US dollars. Similarly, the total costs of injury crashes without installation of sequential warning lights  $C_{Total,Without}^{Injury}$  were estimated to be 77.6 injury crashes/year  $\times$  \$108,600 per injury crash, or \$8.43 million in 2000 US dollars. Based on equation (8), the monetized annual saving from fatal crashes and injury crashes with sequential lights were \$2.36 million and \$0.63 million in 2000 US dollars, respectively. Hence, the total monetized benefits of implementing sequential warning lights were estimated to be \$3.00 million annually in 2000 US dollars, which was equivalent to \$3.65 million in 2010 US dollars using a conservative 2% discount rate.

**TABLE 14 The Result of Parameters**

Fatal Crash	Injury Crash
( $R_{Fatal}$ )	( $R_{Injury}$ )
85.6%	92.5%

**TABLE 15 Freeway and Major Highway Nighttime Work Zone Crashes in Missouri**

	2006	2007	2008	2009	2010	Total
Fatal Crashes	7	1	5	2	7	22
Injury Crashes	149	56	46	63	76	388

**TABLE 16 User Costs of Crashes (year 2000 dollars)**

Type of work zone crash	Average Perceived User Cost	Average Insurance Reimbursement	Net Perceived User Cost
Fatal crashes	3,753,200	29,500	3,723,700
Injury crashes	138,100	29,500	108,600
Property Damage Only	3,900	3,700	200

## Total Costs

Total costs of implementing sequential warning lights ( $C_{Total}$  dollars per year) were computed by adding the total costs of sequential warning lights devices in Missouri ( $C_{Device}$  dollars per year), the costs of batteries ( $C_{Battery}$  dollars per year) and the total labor costs of installing and removing the devices ( $C_{Labor}$  dollars per year).

$$C_{Total} = C_{Device} + C_{Battery} + C_{Labor} ,$$

Based on the information provided by the manufacture of sequential warning lights, the current price of each lamp was approximately \$104 and each lamp consumed the equivalent of \$0.2 of electricity from two batteries every night. According to the MUTCD, each work zone could deploy approximately 20 lights at the taper area. The exact number of lights depends on site characteristics such as the speed limit. The MoDOT work zone schedules from 2010 showed that 1968 nighttime work zones were deployed on freeways in Missouri with an average of 7.6 nights duration per work zone. At most, 109 nighttime work zones were carried out on the same night, thus 109 was the maximum number of sequential light sets required if deployment were desired at all nighttime work zones. Therefore, the total costs of sequential warning lights devices  $C_{Device}$  were estimated to be  $\$104/\text{lights} \times 20 \text{ lights/WZ} \times 109 \text{ WZs}$ , or \$226,760 current US dollars. The total costs of batteries  $C_{Battery}$  were estimated to be  $\$0.2/\text{lights} \times 20 \text{ lights/WZ/night} \times 1968 \text{ WZs} \times 7.6 \text{ nights}$ , or \$59,716 current US dollars.

The labor costs,  $C_{Labor}$ , was computed in two ways to reflect two different strategies of deploying sequential lights. One strategy, used typically in temporary work zones, is to redeploy channelizers each night thus requiring the installation and removal of sequential lights each night. Currently, it is not possible to permanently install sequential lights on channelizers so as to eliminate the installation and removal of lights each time it is deployed. One reason is because channelizers are stacked when transported, and the installation of sequential lights prohibits the stacking of channelizers. Another reason is that the handles of sequential lights are not designed to be used for picking up channelizers which have heavy bases for stability. In the future, perhaps the sequential light function could be designed into the channelizer itself. This will eliminate significant labor costs with deployment and enable channelizers to be stacked. Re-locating batteries to the channelizer base could also help with stability and crash performance. A different strategy is to keep the sequential lights installed on channelizers for the duration of the work zone. With this strategy, channelizers would be left in place or moved to the side of the road when not in use. Thus the sequential lights would be installed and removed only once per work zone.

According to MU's estimates for each work zone, it took 2 workers about 30 minutes to install sequential lights for twenty channelizers and 30 minutes to remove them using manual tools. According to MoDOT's maintenance supervisor, a typical worker salary is approximately \$14 per hour. For the first strategy, the labor costs were estimated as  $\$14/\text{worker-hr} \times 2 \text{ workers} \times (0.5+0.5)\text{hr/WZ/nights} \times 1968\text{WZs} \times 7.6 \text{ nights}$ , or \$418,572 current US dollars. The total costs of implementing sequential warning lights,  $C_{Total}$ , were \$705,008 per year. For the second strategy, the labor costs were estimated as  $\$14/\text{worker-hr} \times 2 \text{ workers} \times (0.5+0.5)\text{hr/WZ/nights} \times 1968\text{WZs} \times 1 \text{ night}$ , or \$55,104 current US dollars. The total costs of implementing strategy two,  $C_{Total}$ , were \$341,580 per year. Since labor could be a major component of cost, an improvement in deployment strategy or an increase in worker efficiency could significantly reduce overall deployment costs. The use of portable power wrenches could decrease labor costs.

Common measures for economic evaluation include benefit-cost ratio, net benefits and cost effectiveness. Such measures are interrelated but serve somewhat different purposes. Using the total benefit amount of \$3.65 million and the total cost amount of \$705,008 or \$341,580, the benefit-cost ratio of deploying sequential lights in Missouri was estimated to be around 5.18 or 10.7. By subtracting the total costs from the total benefits, the net annual benefits were computed to be \$2.94 or \$3.31 million. Using the crash ratios from **TABLE 14** and the average annual nighttime work zones crashes from **TABLE 15**, the annual crash reductions were estimated to be 0.634 fatality/year and 5.84 injury/year. The cost effectiveness of sequential lights was then estimated by dividing the total cost by the expected crash reductions. Assuming an equivalency of 34.3 injuries to a single fatality, the cost effectiveness was estimated to be \$25,566/injury or \$12,382/injury.

Some examples of benefit-cost ratios for other low-cost safety counter-measures are presented here as background information. Centerline rumble strip benefit-cost ratios were estimated to be around 0.99 to 24.88 depending on the ADT (Carlson and Miles, 2003). The benefit-cost ratios for raised pavement markers on two lane roads were estimated to be around 14.49 to 25.51 (Neuman et al., 2003). And the benefit-cost ratios of edgelines on two-lane roads were estimated to be around 8.6 to 85.7 depending on traffic volumes (Miller, 1992).

## CONCLUSION

The evaluation of sequential warning lights was based on three measures of safety performance: vehicle speed and speed variability, taper conflict, and closed lane merge location (i.e. closed lane occupancy). Although sequential lights caused a small increase in speed variance, it caused a significant decrease in vehicle speed and an increase in driver compliance at nighttime work zones. The cumulative speed distributions showed sequential lights reduced the speeds of both passenger cars and trucks at both rural and urban work zones for all speed ranges. That effect was more pronounced at the urban work zone than at rural work zones. All speed results were analyzed statistically.

Vehicle position data at the near taper view demonstrated that sequential lights prevented a significant proportion of vehicles from late taper merges at rural work zone taper. But at the urban work zone, the percentage of late taper merge increased with sequential lights. One possible explanation was that there was a subset of more aggressive drivers who merged near the taper because the edge of the taper became more identifiable with sequential lights. This late merging and last minute over-taking behavior was more common in the urban environment because of the higher amount of traffic.

Despite the aforementioned issue at the taper, the overall merging behavior improved with sequential lights. In general, vehicles that merge earlier are at a lower risk of a merging conflict because there is more time to react to the closed lane. The use of sequential lights produced a significant shift in the proportion of total vehicle merges from near the taper to farther away from the taper. In particular, sequential lights had a larger effect on trucks than passenger cars, and on rural work zones than the urban work zone. The effect at the urban work zone was possibly not realized because high traffic volume left few gaps for early merges.

The second objective of this report was to evaluate the monetized benefits and costs of sequential lights. No crash analysis was performed because there was no significant crash data associated with sequential lights deployment. However, a crash model was used to estimate improvements in safety from the reduction in speeds. The deployment costs were computed in two ways because the installation and removals labor costs were so significant. Based on Nilsson's power model and MoDOT's work zone crash data, the total annual benefits was estimated to be \$3.65 million and total annual costs was estimated to be \$705,008 or \$341,580. These estimates assumed that sequential lights were deployed on all nighttime interstates and major highway work zones in Missouri. The resulting benefit-cost ratio was around 5 or 10 and the cost effectiveness was around \$25,000 per injury or \$12,000 per injury.

In terms of operations, there were no synchronization problems that were observed. The sequential lights were observed in operation in in-house lab tests and in three field tests. For the in-house

test, lights were purposely re-arranged and their communications interrupted by inserting barriers between lights. The lights were able to re-synchronize successfully after the removal of disruptions.

Crash testing was outside the scope of this project, thus crash testing was not performed on channelizers with sequential lights attached. The manufacturer indicates that sequential lights are NCHRP 350 compliant.

In summary, sequential lights appear to be effectiveness for improving safety at nighttime work zones by clearly delineating the taper area. They are more effective for trucks and at rural work zones as compared to passenger cars and at urban work zones. A small percentage of drivers became more aggressive with overtaking at the taper, because the taper became more visible. In general, sequential lights caused vehicles to merge further upstream from the taper. Because labor is a major component of sequential lights deployment, improvements in design could reduce agency deployment costs significantly.

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